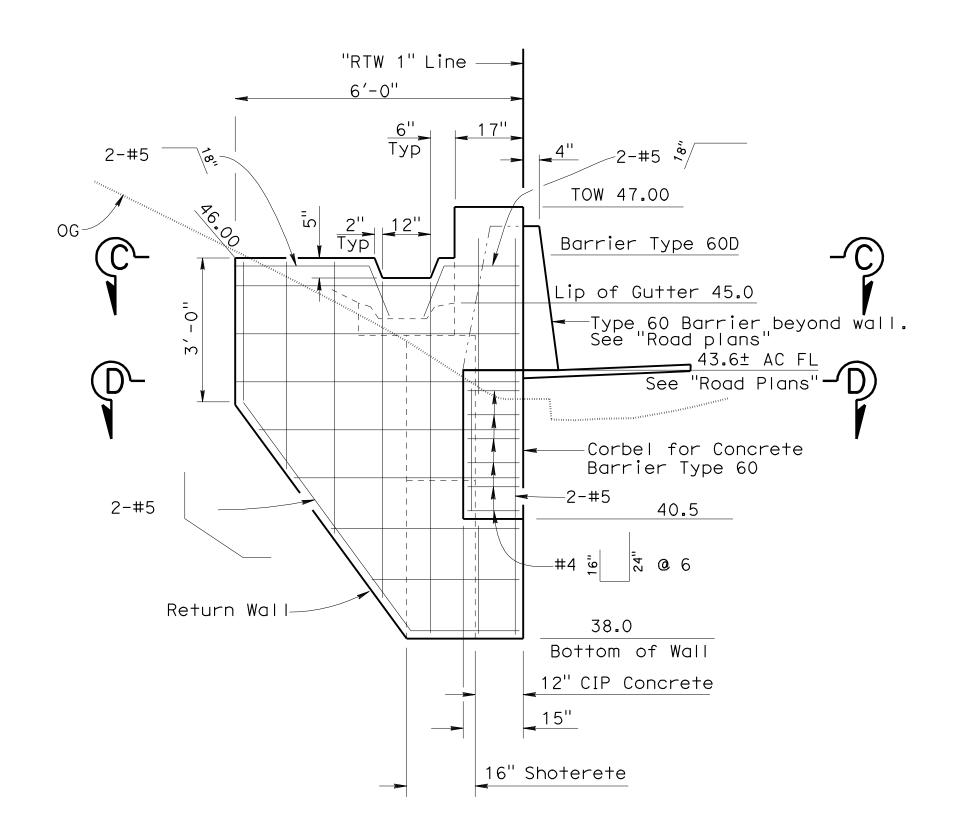
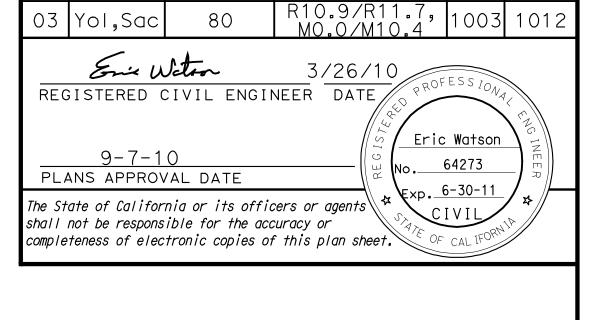


RETURN WALL ELEVATION AT BEGIN WALL

1/2" = 1'-0"





Notes:

 Reinforcement for barrier, shotcrete panels, and transition block is omitted for clarity.

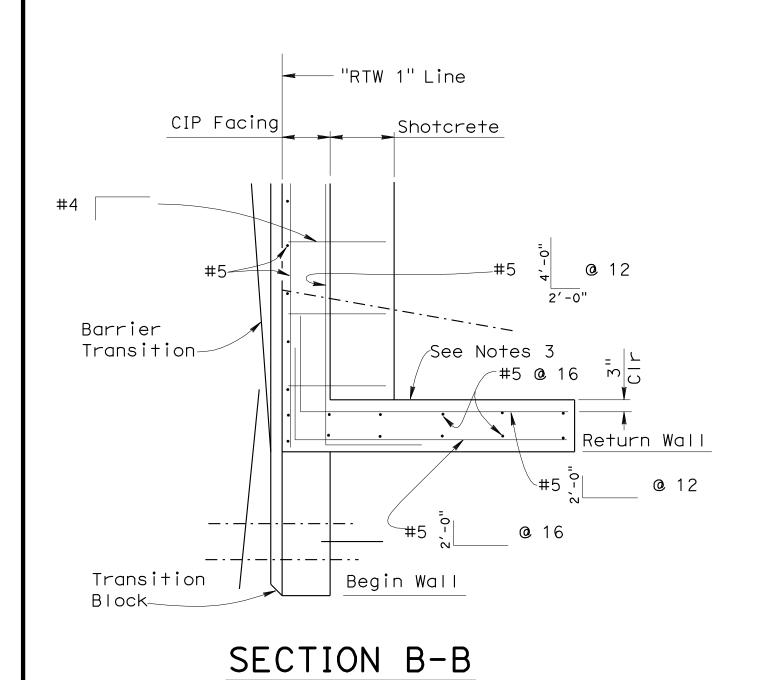
DIST

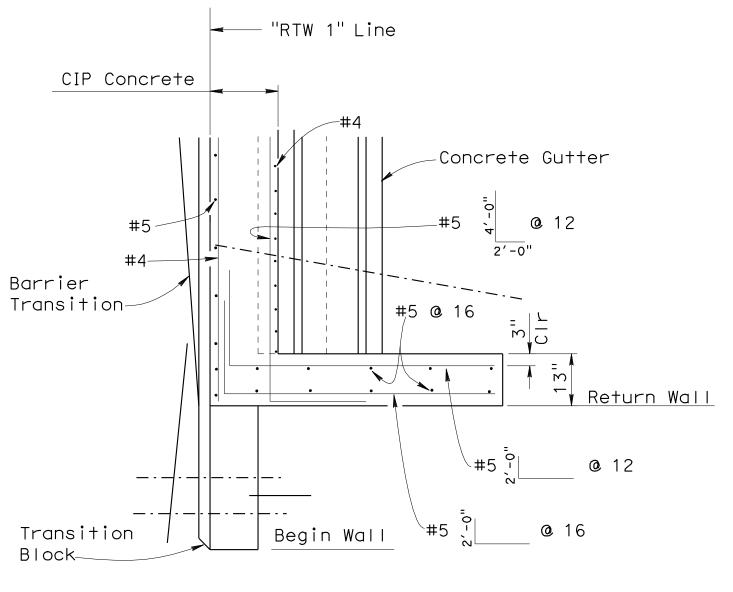
COUNTY

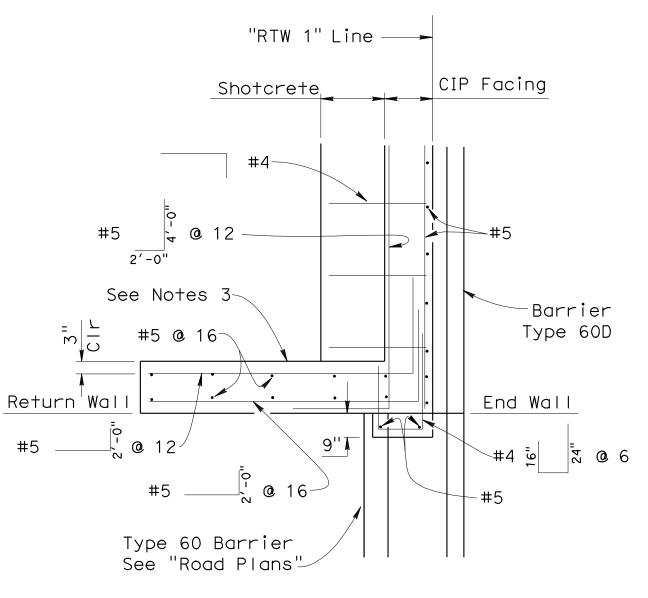
- The contractor shall verify all controlling field dimensions before ordering or fabricating any material.
- 3. Cast the inside face of the portion of the return wall below the top of the shotcrete panel against undisturbed earth.
- 4. Form the inside face of the return wall above the level of the ditch.

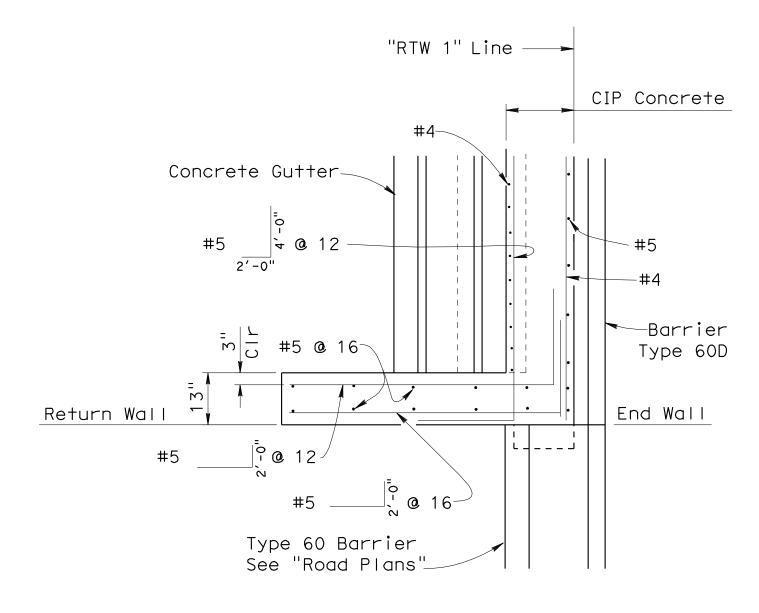
RETURN WALL ELEVATION AT END WALL

<mark>1/2" = 1'-0"</mark>







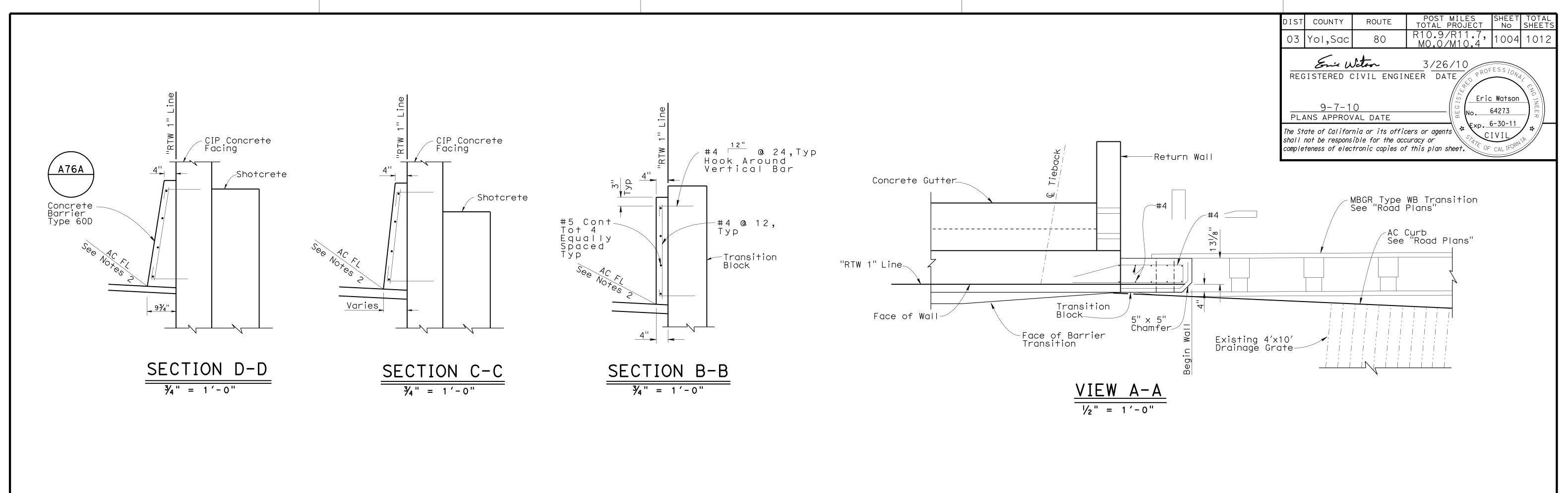


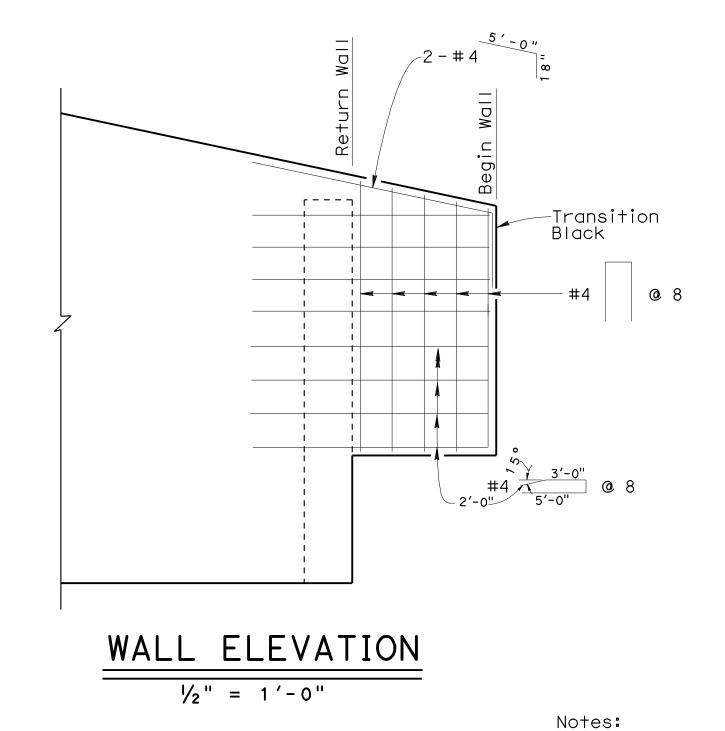
SECTION A-A

 $\frac{SECTION D-D}{\frac{1}{2}" = 1'-0"}$

 $\frac{\text{SECTION C-C}}{\frac{1}{2}" = 1'-0"}$

	DESIGN Mark Simonsen DETAILS BY Jinrong Zhou QUANTITIES BY Yihwin Huang	CHECKED Daniel Sessions CHECKED Daniel Sessions CHECKED Jie Tang	STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION	DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN DESIGN BRANCH	BRIDGE NO. 24E0006 POST MILE 9.4	RETAINING WALL 1 DETAILS NO. 4
STRUCTURES DESIGN DETAIL SHEET (ENGLISH) (REV. 10/25/05)	<u> </u>	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU 03 EA 3797U1	DISREGARD PRINTEARLIER REVISIO	
				FILE => 24e0006-g-rwd+04.dgn		



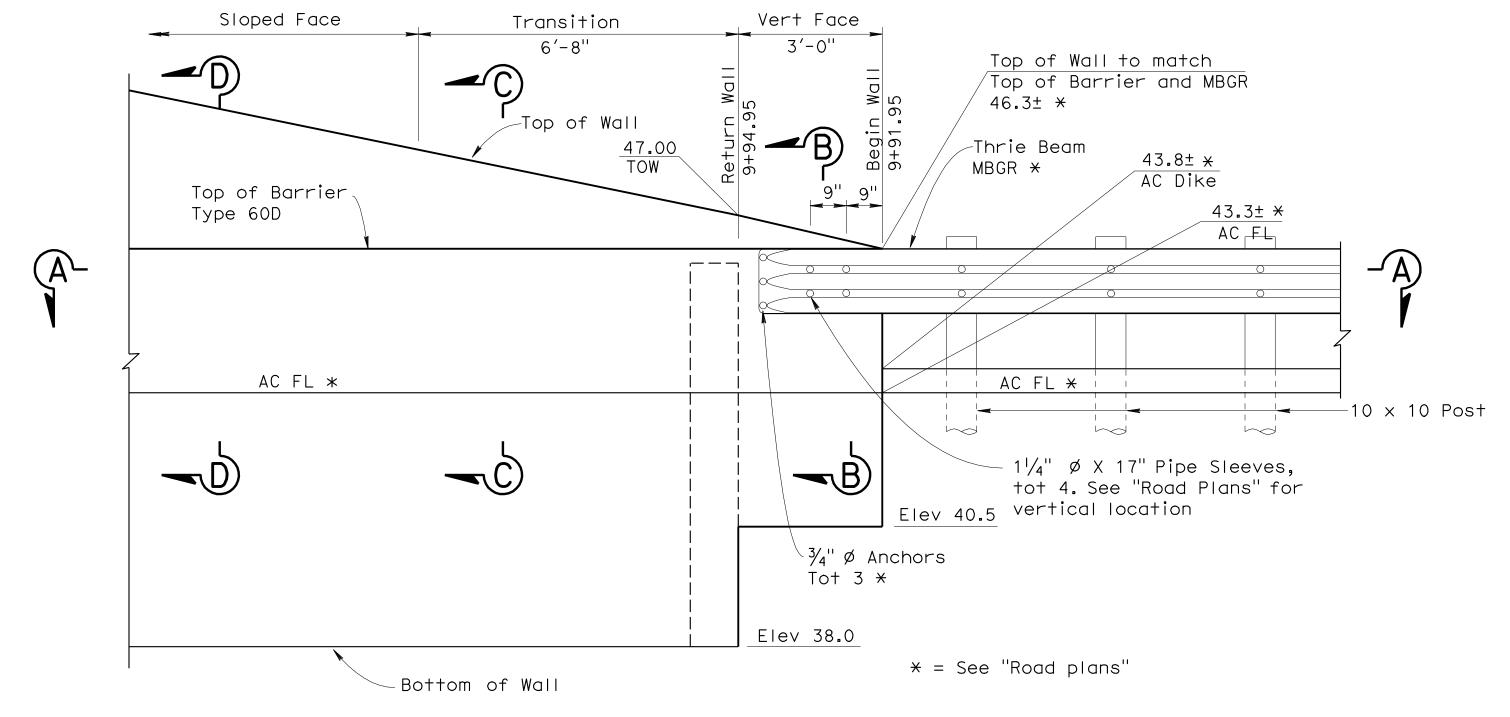


1. Wall reinforcing omitted for clarity

2. Place Type 60 Concrete Barrier on finished AC Paving. See "Road Plans"

NOTE:
THE CONTRACTOR SHALL VERIFY ALL
CONTROLLING FIELD DIMENSIONS
BEFORE ORDERING OR FABRICATING
ANY MATERIAL.

STRUCTURES DESIGN DETAIL SHEET (ENGLISH) (REV. 10/25/05)

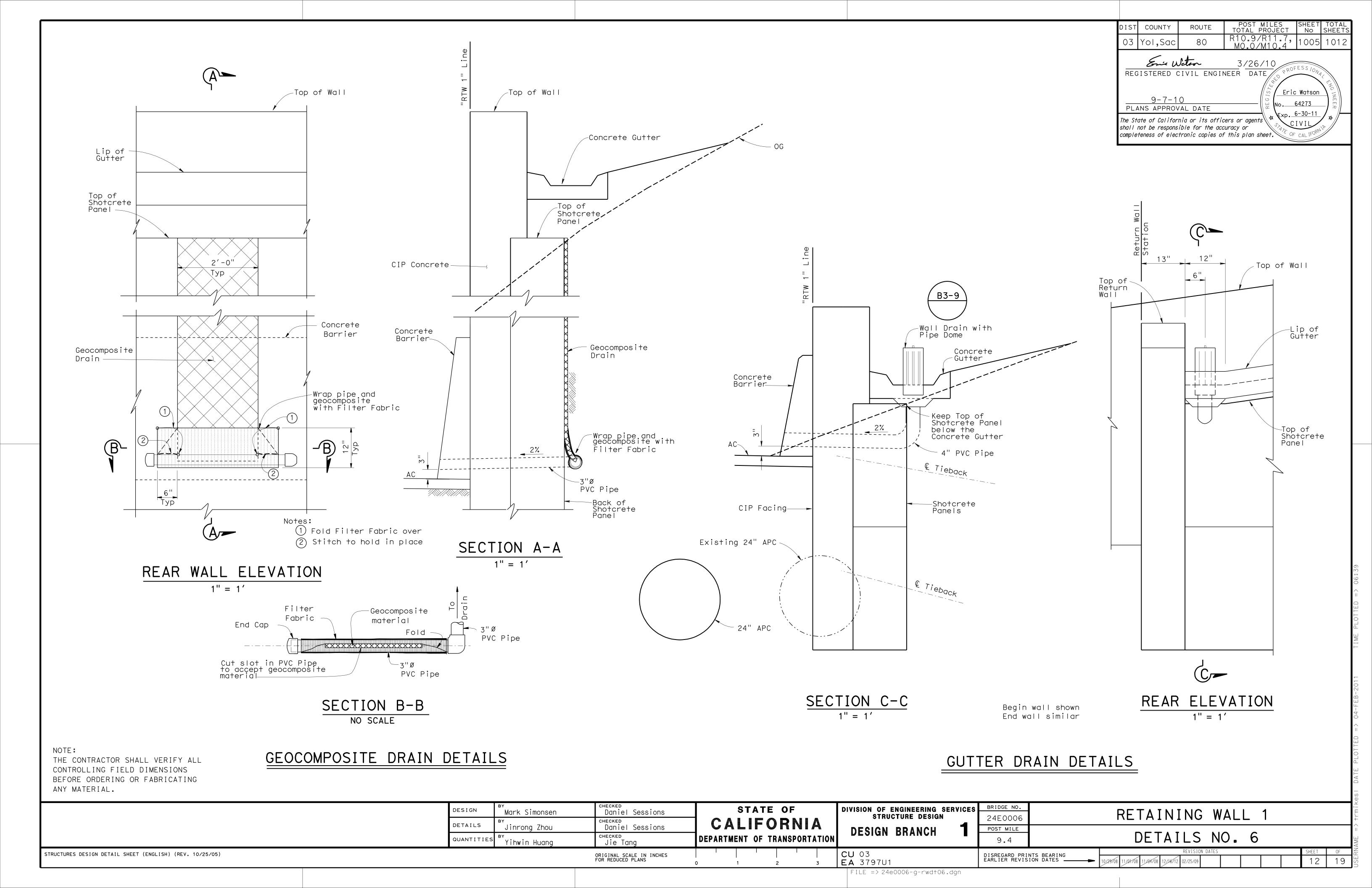


ELEVATION AT BEGIN WALL FACING THE WALL

1/2" = 1'-0"

DESIGN Mark Simonsen DETAILS Linrang 7hour	Daniel Sessions	STATE OF CALIFORNIA	DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN	24E0006	RETAINING WAL 1				
QUANTITIES BY Yihwin Huang	Daniel Sessions CHECKED Jie Tang	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	9.4 DETAILS NO. 5					
	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU 03 EA 3797U1	DISREGARD PRIN EARLIER REVIS	NTS BEARING ION DATES	REVISION DATES 10/27/08 11/01/08 11/04/08 12/17/08 02/25/09	SHEET 1 1	0F	

FILE => 24e0006-g-rwd+05.dgn



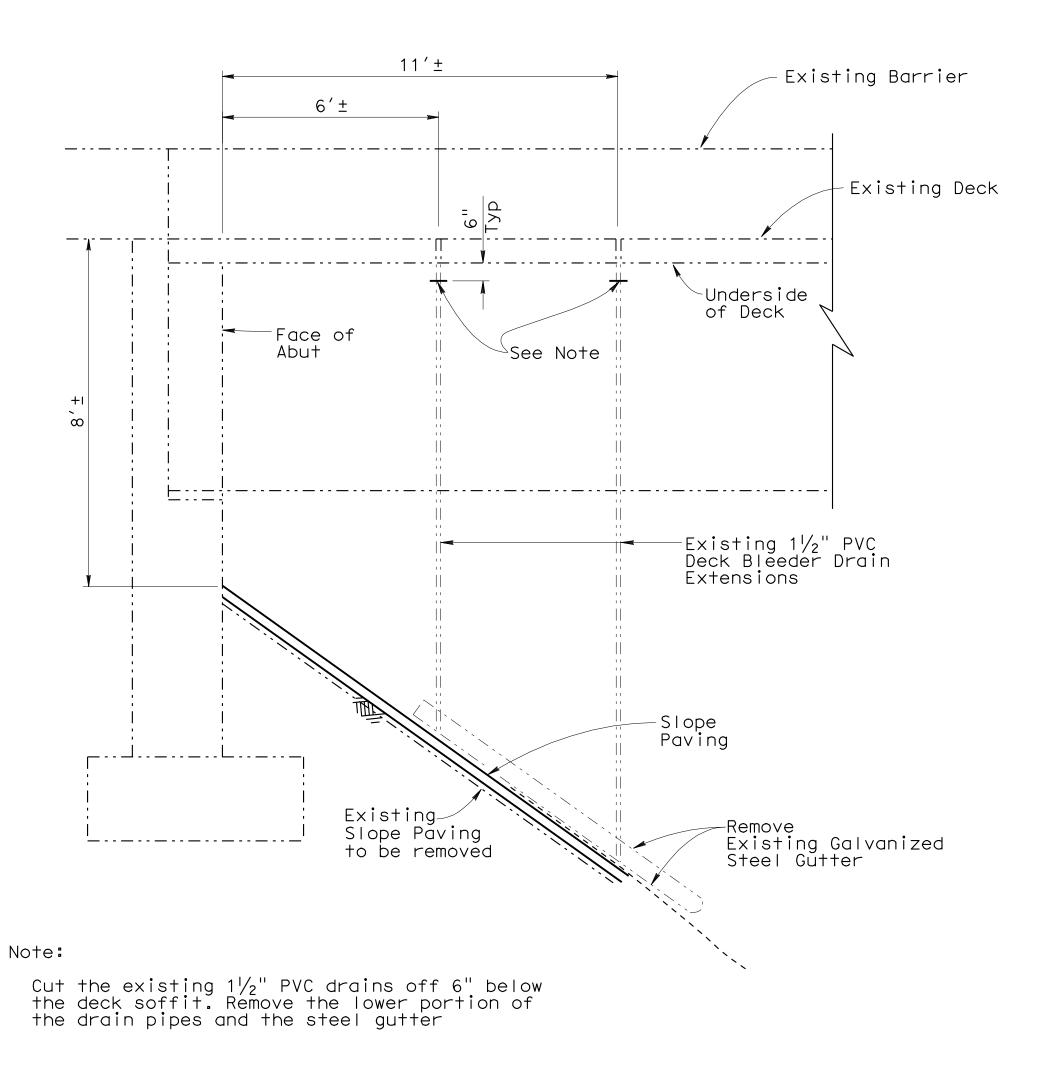
DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

O3 YOI, Sac 80 R10.9/R11.7, 1006 1012

REGISTERED CIVIL ENGINEER DATE

9-7-10
PLANS APPROVAL DATE

The State of California or its officers or agents shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet.



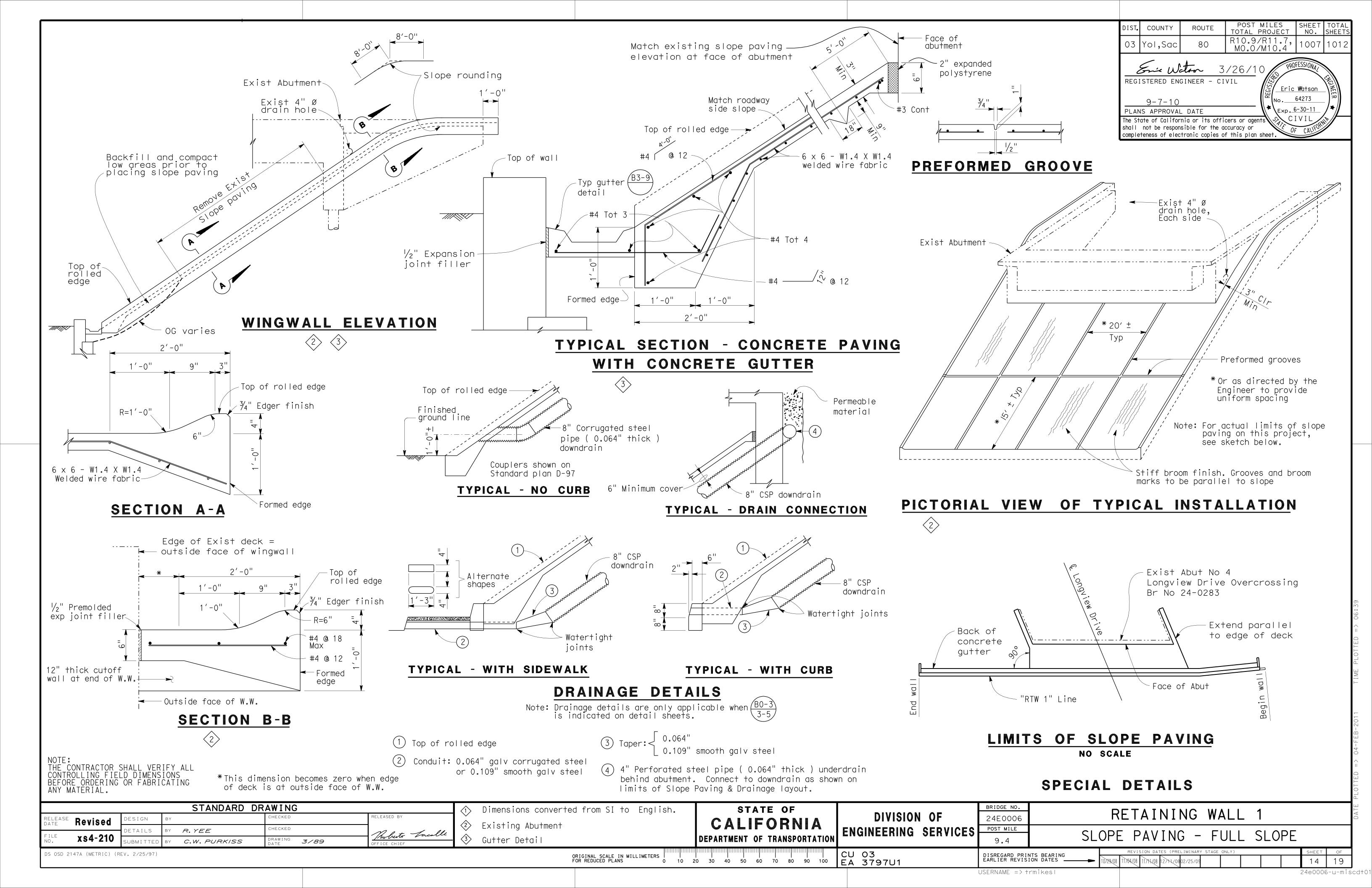
DECK BLEEDER DRAIN MODIFICATIONS

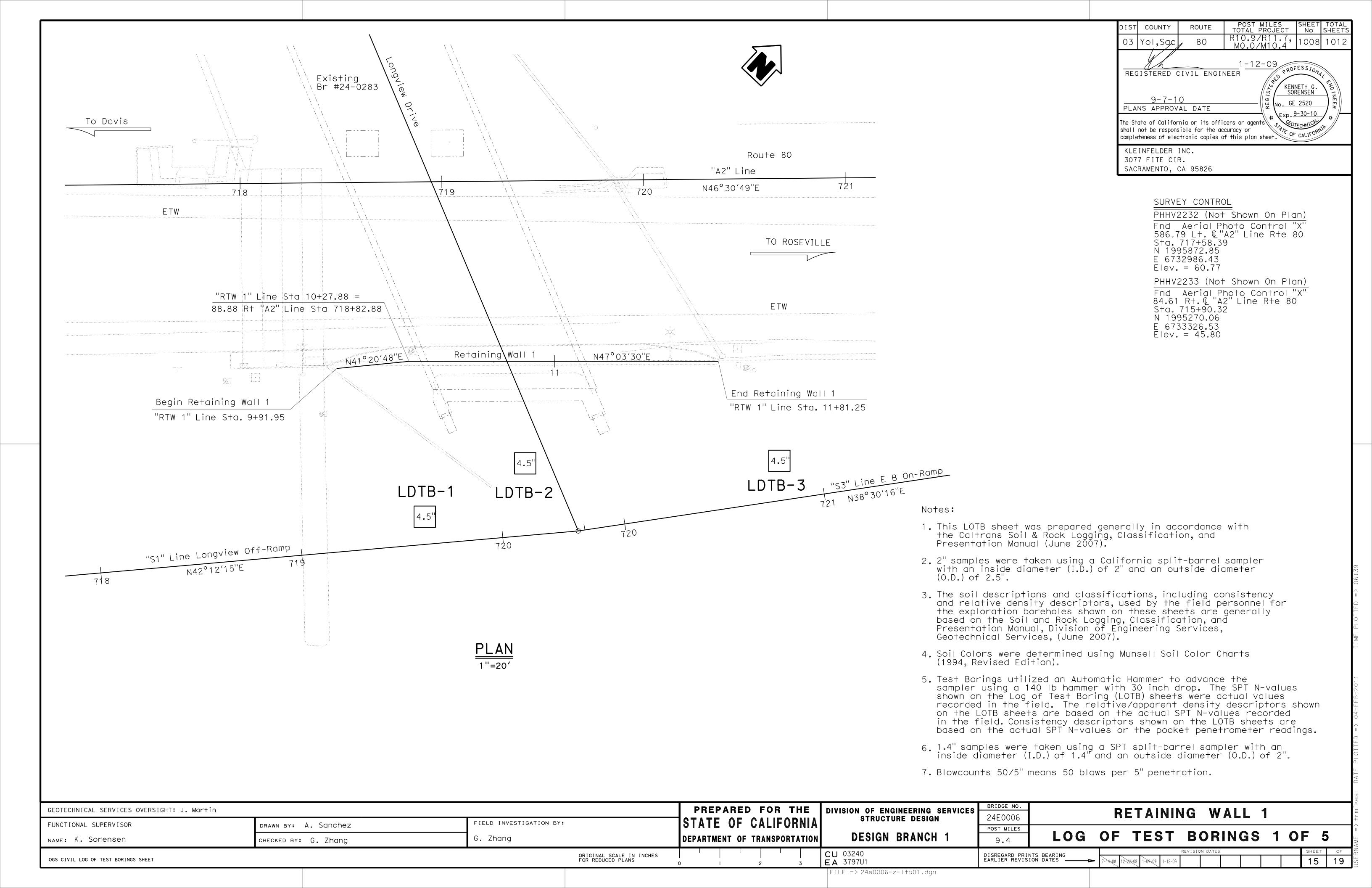
 $\frac{3}{8}$ " = 1'-0"

NOTE:
THE CONTRACTOR SHALL VERIFY ALL
CONTROLLING FIELD DIMENSIONS
BEFORE ORDERING OR FABRICATING
ANY MATERIAL.

	DESIGN	Mark Simonsen	CHECKED Daniel Sessions	V 1111 = V1			DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN		RETAINING WALL 1
	DETAILS	BY Jinrong Zhou	CHECKED Daniel Sessions	CAI	LIFO	RNIA	DESIGN BRANCH	24E0006 POST MILE	
	QUANTITIES	BY Yihwin Huang	checked Jie Tang	DEPARTME	NT OF TRA	ANSPORTATION	DESIGN BRANCH	9.4	DETAILS NO. 7
STRUCTURES DESIGN DETAIL SHEET (ENGLISH) (REV. 10/25/05)			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS) 1	2	3	CU 03 EA 3797U1	DISREGARD PRI EARLIER REVIS	

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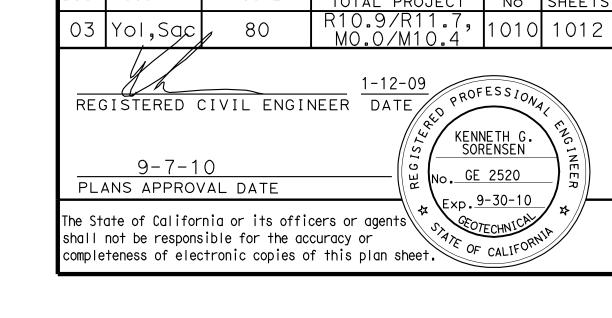
DIST COUNTY R10.9/R11.7, 1009 03 Yol, Sac NOTE: This LOTB sheet was prepared generally in accordance with the Caltrans Soil & Rock Logging, Classification, and Presentation Manual (June 2007). FOR PLAN VIEW AND ADDITIONAL NOTES, SEE "LOG OF TEST BORINGS" SHEET 1 OF 5 1-12-09 REGISTERED CIVIL ENGINEER KENNETH G. SORENSEN 9-7-10 lo. GE 2520 PLANS APPROVAL DATE Exp. 9-30-10 The State of California or its officers or agents shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. KLEINFELDER INC. 3077 FITE CIR. SACRAMENTO, CA 95826 120 120 110 110 100 100 90 90 166.0'R+ S+a. 718+90.0 140.0' Rt Sta. 720+66.0 "A2" Line "A2" Line 80 80 LDTB-B1 LDTB-B2 LDTB-B3 70 70 Elev 70.55 Elev 70.55 Elev 70.55 ASPHALT CONCRETE; (about 1-1/2 inches thick). SANDY lean CLAY (CL); stiff; brown; moist [FILL]. SANDY lean CLAY (CL); stiff; reddish brown; moist; 15 1.4 9 | 1.4 / fine SAND [FILL]. AGGREGATE BASE; (about 12 inches thick). CLAYEY SAND (SC); loose; reddish brown; moist; fine 8 1.4 8 1.4 19 1.4 — SILT (ML); very stiff; brown; moist [FILL]. 60 60 — Lean CLAY (CL); very stiff; brown; moist [FILL]. SILTY SAND (SM); medium dense; reddish brown; 4 1 4 22 1.4 17 1.4 moist; fine SAND. SILT (ML); very stiff; brown; moist. SILT (ML); hard; reddish brown; moist; moderately cemented, very thin calcite veins. 50/6 1.4 SILTY SAND (SM); very dense; brown; moist. 25 1.4 72 1.4 Lean CLAY (CL); very stiff; brown; moist [FILL]. Lean CLAY (CL); hard; brown; moist. 50 50 SILTY SAND (SM); medium dense; reddish brown; 15 1.4 36 1.4 Lean CLAY (CL); very stiff; brown; moist. 23 2.0 SILTY SAND (SM); medium dense; brown; moist; fine moist; fine SAND. to medium SAND [FILL]. SILT (ML); very stiff; brown; moist. 29 2.0 36 1.4 CLAYEY SAND with GRAVEL (SC); dense; yellowish 23 1.4 SILT (ML); very stiff; brown; moist. brown; moist; fine SAND; rounded fine gravel. 40 40 SILTY SAND (SM); medium dense; olive; moist; fine 15 1.4 18 1.4 23 1.4 -Stiff. SANDY SILT (ML); very stiff; brown; moist; fine to medium SAND. 23 1.4 31 2.0 18 1.4 SILTY SAND (SM); medium dense; brown; moist; fine SILTY SAND (SM); dense; brown; moist. -Laminated lenses of sand and silt. to medium SAND; mica present. 30 30 Poorly graded SAND with GRAVEL (SP); medium dense; moist; fine SAND. -Medium dense. 30 1.4 22 1.4 21 1.4 Poorly graded SAND (SP); dense; brown; moist; fine to coarse SAND. Poorly graded SAND (SP); medium dense; brown; 57 2.0 25 1.4 41 1.4 -Laminations. moist; medium to coarse SAND. 20 20 SILTY SAND (SM); very dense; brown; moist; fine SILT (ML); very stiff; brown; moist. 30 1.4 -dense. 25 1.4 37 1.4 Poorly graded SAND (SP); medium dense; brown; moist; fine to coarse SAND; mica present. - SILTY SAND (SM); medium dense; brown; moist; fine to coarse SAND. SILT (ML); very stiff; brown; moist; mica present. 10 7-25-07 7-25-07 10 7-18-07 Terminated at Elev 19.05 Terminated at Elev 19.05 Terminated at Elev 19.05 ER; = 82%ER; = 68% $ER_{i} = 82\%$ 0 0 -10-10 PROF I LE 720+00 717+00 718+00 719+00 721+00 722+00 HOR. 1"=20' VER. 1"=10' BRIDGE NO. PREPARED FOR THE DIVISION OF ENGINEERING SERVICES GEOTECHNICAL SERVICES OVERSIGHT: J. Martin RETAINING WALL 1 24E0006 STRUCTURE DESIGN STATE OF CALIFORNIA FIELD INVESTIGATION BY: FUNCTIONAL SUPERVISOR DRAWN BY: A. Sanchez POST MILES OF TEST BORINGS 2 OF 5 LOG **DESIGN BRANCH 1** G. Zhang DEPARTMENT OF TRANSPORTATION NAME: K. Sorensen 9.4 CHECKED BY: G. Zhang CU 03240 ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES EA 3797U1 16 | 19 OGS CIVIL LOG OF TEST BORINGS SHEET FILE = > 24e0006-z-1+b02.dgn

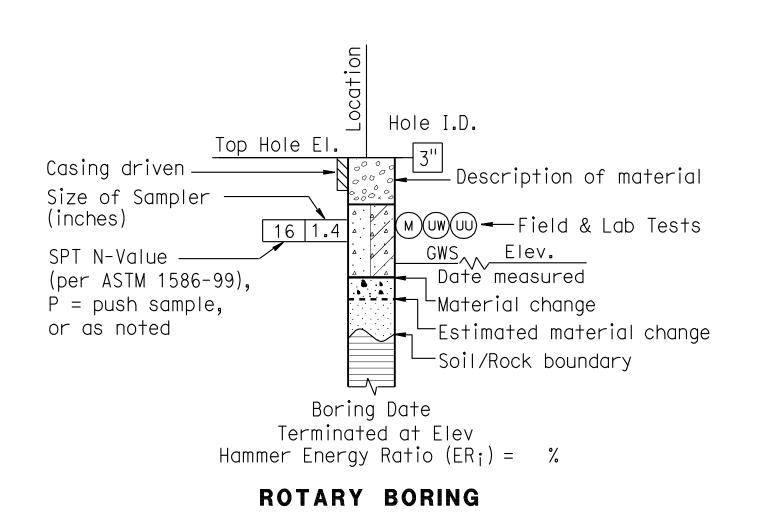
CEMENTATION						
Description	Criteria					
Weak	Crumbles or breaks with handling or little finger pressure.					
Moderate	Crumbles or breaks with considerable finger pressure.					
Strong	Will not crumble or break with finger pressure.					

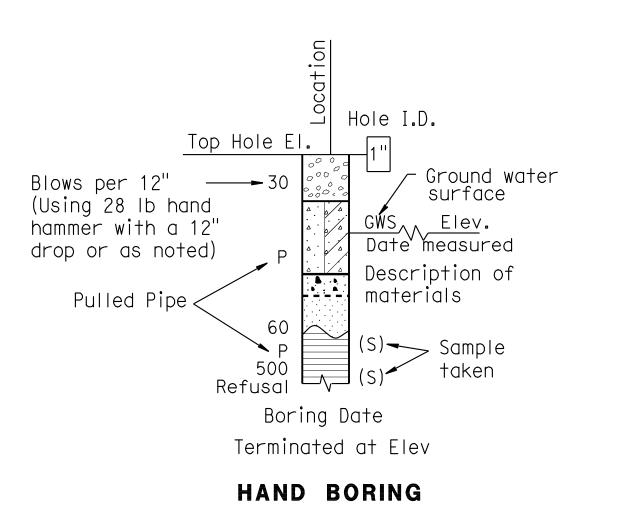
	BOREHOLE IDENTIFICATION					
Symbol	Hole Type	Description				
Size	А	Auger Boring				
Size	R P	Rotary drilled boring Rotary percussion boring (air)				
Size	R	Rotary drilled diamond core				
Size	HD HA	Hand driven (1-inch soil tube) Hand Auger				
•	D	Dynamic Cone Penetration Boring				
	CPT	Cone Penetration Test (ASTM D 5778-95)				
[]	0	Other				
Note: Size in inches.						

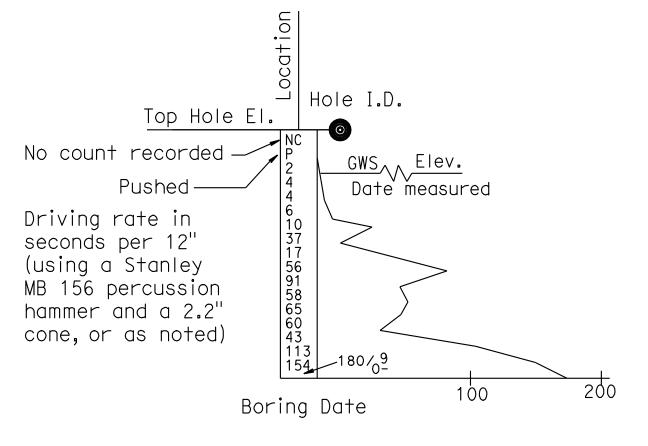
	CONSISTENCY OF COHESIVE SOILS						
Description	Unconfined Compressive Strength (tsf)	Pocket Penetrometer Measurement (tsf)	Torvane Measurement (tsf)	Field Approximation			
Very Soft	< 0.25	< 0.25	< 0.12	Easily penetrated several inches by fist			
Soft	0.25 to 0.50	0.25 to 0.50	0.12 to 0.25	Easily penetrated several inches by thumb			
Medium Stiff	0.50 to 1.0	0.50 to 1.0	0.25 to 0.50	Penetrated several inches by thumb with moderate effort			
Stiff	1 to 2	1 to 2	0.50 to 1.0	Readily indented by thumb but penetrated only with great effort			
Very Stiff	2 to 4	2 to 4	1.0 to 2.0	Readily indented by thumbnail			
Hard	> 4.0	> 4.0	> 2.0	Indented by thumbnail with difficulty			

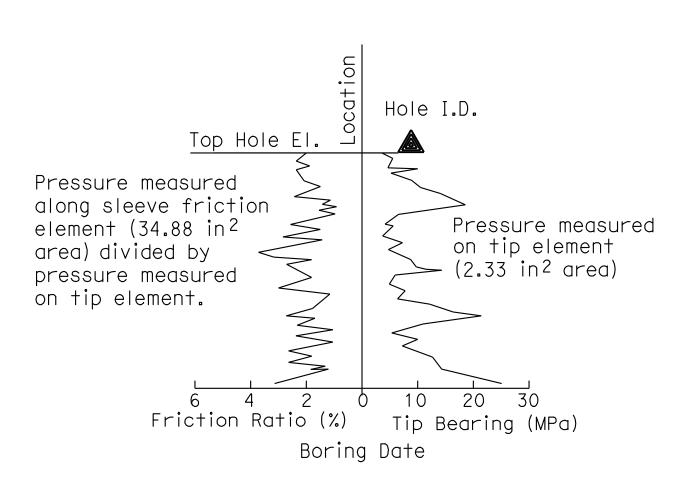
PLASTICITY OF FINE-GRAINED SOILS						
Description	Criteria					
Nonplastic	A 1/8-inch thread cannot be rolled at any water content.					
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.					
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit.					
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.					











DYNAMIC CONE PENETRATION BORING

CONE PENETRATION TEST (CPT) SOUNDING

			EPARED FOR THE DIVISION OF ENGINEERING SERVICES		BRIDGE NO. 24E0006			ING WALL	.L 1		
FUNCTIONAL SUPERVISOR: K. Sorensen	PREPARED BY A. Sanchez CHECKED BY G. 7bana		STATE OF C		BEALAN BRANAU 4	POST MILE	LOG	OF TEST		3 of 5	
GS LOTB SOIL LEGEND	G. Zhang	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	DEFAITIMENT OF T	2 3	CU 03240 EA 3797U1	9.4 DISREGARD PRIN EARLIER REVISI		7-14-08 12-22-08 1-9-09 1-12-	REVISION DATES	SHEET	0F 19

FILE => 24e0006-z-l+b03.dgn

		GROUP SYMBOLS	AND	NAME	S
•	c/Symbol	Group Names	Graphic	c/Symbol	Group Names
	GW	Well-graded GRAVEL Well-graded GRAVEL with SAND Poorly graded GRAVEL		CL	Lean CLAY Lean CLAY with SAND Lean CLAY with GRAVEL SANDY lean CLAY SANDY lean CLAY with GRAVEL
	GP GW-GM	Poorly graded GRAVEL with SAND Well-graded GRAVEL with SILT and SAND			GRAVELLY lean CLAY GRAVELLY lean CLAY with SAND SILTY CLAY SILTY CLAY with CRAVEL
	GW-GC	Well-graded GRAVEL with SILT and SAND Well-graded GRAVEL with CLAY (or SILTY CLAY) Well-graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)		CL-ML	SILTY CLAY with GRAVEL SANDY SILTY CLAY SANDY SILTY CLAY with GRAVEL GRAVELLY SILTY CLAY GRAVELLY SILTY CLAY with SAND
	GP-GM	Poorly graded GRAVEL with SILT Poorly graded GRAVEL with SILT and SAND Poorly graded GRAVEL with CLAY (or SILTY CLAY)	_	ML	SILT SILT with SAND SILT with GRAVEL SANDY SILT SANDY SILT with GRAVEL
	GP-GC	(or SILTY CLAY) Poorly graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND) SILTY GRAVEL			GRAVELLY SILT GRAVELLY SILT with SAND ORGANIC lean CLAY
	GM	SILTY GRAVEL with SAND CLAYEY GRAVEL		OL	ORGANIC lean CLAY with SAND ORGANIC lean CLAY with GRAVEL SANDY ORGANIC lean CLAY SANDY ORGANIC lean CLAY with GRAVEL
	GC	CLAYEY GRAVEL with SAND SILTY, CLAYEY GRAVEL			GRAVELLY ORGANIC lean CLAY GRAVELLY ORGANIC lean CLAY with SAND ORGANIC SILT
	GC-GM	SILTY, CLAYEY GRAVEL with SAND Well-graded SAND		OL	ORGANIC SILT with SAND ORGANIC SILT with GRAVEL SANDY ORGANIC SILT SANDY ORGANIC SILT with GRAVEL
	SW SP	Well-graded SAND with GRAVEL Poorly graded SAND			GRAVELLY ORGANIC SILT GRAVELLY ORGANIC SILT with SAND Fat CLAY Fat CLAY with SAND
	SW-SM	Poorly graded SAND with GRAVEL Well-graded SAND with SILT Well-graded SAND with SILT and GRAVEL		СН	Fat CLAY with GRAVEL SANDY fat CLAY SANDY fat CLAY with GRAVEL GRAVELLY fat CLAY GRAVELLY fat CLAY with SAND
	SW-SC	Well-graded SAND with CLAY (or SILTY CLAY) Well-graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)		MH	Elastic SILT Elastic SILT with SAND Elastic SILT with GRAVEL SANDY elastic SILT
	SP-SM	Poorly graded SAND with SILT Poorly graded SAND with SILT and GRAVEL Poorly araded SAND with CLAY			SANDY elastic SILT with GRAVEL GRAVELLY elastic SILT GRAVELLY elastic SILT with SAND ORGANIC fat CLAY
	SP-SC SM	Poorly graded SAND with CLAY (or SILTY CLAY) Poorly graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL) SILTY SAND		ОН	ORGANIC fat CLAY with SAND ORGANIC fat CLAY with GRAVEL SANDY ORGANIC fat CLAY SANDY ORGANIC fat CLAY with GRAVEL GRAVELLY ORGANIC fat CLAY
	SC	SILTY SAND with GRAVEL CLAYEY SAND CLAYEY SAND with GRAVEL			GRAVELLY ORGANIC fat CLAY with SAND ORGANIC elastic SILT ORGANIC elastic SILT with SAND ORGANIC elastic SILT with GRAVEL
	SC-SM	SILTY, CLAYEY SAND SILTY, CLAYEY SAND with GRAVEL		ОН	SANDY ORGANIC elastic SILT SANDY ORGANIC elastic SILT with GRAVEL GRAVELLY ORGANIC elastic SILT GRAVELLY ORGANIC elastic SILT with SAND
	РТ	PEAT		OL/OH	ORGANIC SOIL ORGANIC SOIL with SAND ORGANIC SOIL with GRAVEL SANDY ORGANIC SOIL SANDY ORGANIC SOIL with GRAVEL
		COBBLES and BOULDERS BOULDERS			GRAVELLY ORGANIC SOIL GRAVELLY ORGANIC SOIL with SAND

FIELD AND LABORATORY TESTING

- (C) Consolidation (ASTM D 2435)
- (CL) Collapse Potential (ASTM D 5333)
- (CP) Compaction Curve (CTM 216)
- CR) Corrosivity Testing (CTM 643, CTM 422, CTM 417)
- CU Consolidated Undrained Triaxial (ASTM D 4767)
- (DS) Direct Shear (ASTM D 3080)
- (EI) Expansion Index (ASTM D 4829)
- M) Moisture Content (ASTM D 2216)
- OC) Organic Content-% (ASTM D 2974)
- P Permeability (CTM 220)
- (PA) Particle Size Analysis (ASTM D 422)
- PI Plasticity Index (AASHTO T 90) Liquid Limit (AASHTO T 89)
- (PL) Point Load Index (ASTM D 5731)
- (PM) Pressure Meter
- (PP) Pocket Penetrometer
- R Value (CTM 301)
- (SE) Sand Equivalent (CTM 217)
- (SG) Specific Gravity (AASHTO T 100)
- SL) Shrinkage Limit (ASTM D 427)
- (SW) Swell Potential (ASTM D 4546)
- (TV) Pocket Torvane
- Unconfined Compression-Soil
 (ASTM D 2166)
 - Unconfined Compression-Rock (ASTM D 2938)
- Unconsolidated Undrained Triaxial (ASTM D 2850)
- UW) Unit Weight (ASTM D 4767)
- VS) Vane Shear (AASHTO T 223)

FILE => 24e0006-z-l+b04.dgn

DIST	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No	SHEETS	
03	Yol,Sac	80	R10.9/R11.7, M0.0/M10.4	1011	1012	
REGISTERED CIVIL ENGINEER DATE PROFESSIONAL KENNETH G. SORFNSEN						
PLA	$\frac{9-7-10}{\text{PLANS APPROVAL DATE}} \left(\frac{9}{\text{No.}} \left(\frac{\text{GE 2520}}{\text{No.}} \right) \right) \left(\frac{9}{\text{Mo.}} \right)$					
The State of California or its officers or agents shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. EXP. 9-30-10 FORMING OF CALIFORNIA OF CALIFORNIA						

APPARENT DENSIT	Y OF COHESIONLESS SOILS					
Description	SPT N ₆₀ (Blows / 12 inches)					
Very loose	0 - 4					
Loose	5 - 10					
Medium Dense	11 - 30					
Dense	31 - 50					
Very Dense	> 50					

MOISTURE					
Description	Criteria				
Dry	Absence of moisture, dusty, dry to the touch				
Moist	Damp but no visible water				
We†	Visible free water, usually soil is below water table				

PERCENT OR PROPORTION OF SOILS								
Description	Criteria							
Trace	Particles are present but estimated to be less than 5%							
Few	5 to 10%							
Little	15 to 25%							
Some	30 +o 45%							
Mostly	50 to 100%							

PARTICLE SIZE								
Des	scription	Size						
Boulder		> 12"						
Cobble		3" to 12"						
Cravol	Coarse	3/4" to 3"						
Gravel	Fine	No. 4 to 3/4"						
Sand	Coarse	No. 10 to No. 4						
	Medium	No. 40 to No. 10						
	Fine	No. 200 to No. 40						

GEOTECHNICAL SERVICES OVERSIGHT: J. Martin				DIVISION OF ENGINEERING SERVICES	BRIDGE NO.	RETAINING WALL 1					
FUNCTIONAL SUPERVISOR: K. Sorensen	PREPARED BY	A. Sanchez		STATE OF CALIFORNIA		24E0006					
	CHECKED BY	G. Zhang		DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH 1	9.4	LOG	OF	TEST	BORINGS	4 of 5
GS LOTB SOIL LEGEND			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS		CU 03240	DISREGARD PRIN EARLIER REVIS	NTS BEARING ION DATES	7-14-08 12-95	RE	EVISION DATES	SHEET OF 18 19

